

## DAfStb H. 617 - Part 4.1:

### Attachment 4.1-1: Notation and Formulary for the ACI-DAfStb Shear Database vsw-RC-A for the evaluation of reinforced concrete beams with stirrups subjected to point loads

<u>Nr. (no.)</u>	running number,
<u>Lit. (Author)</u>	reference: author, year
<u>Bez. (Test specimen)</u>	specimen as named by author
<u>Einheiten (Units):</u> dual input in Imperial units or SI - units; Imp. units are converted into SI-units, and all calculations in SI-units.	

#### Querschnittswerte (section properties)

b	b	[in → mm]	width of flange
bw	$b_w$	[in → mm]	width of web
h	h	[in → mm]	height of beam
hf	$h_f$	[in → mm]	height of flange
hh,top	$h_{h,top}$	[in → mm]	height of top haunch
hw	$h_w$	[in → mm]	height of web
hft	$h_{ft}$	[in → mm]	height of tension flange
hh,bot	$h_{h,bot}$	[in → mm]	height of bottom haunch
bft	$b_{ft}$	[in → mm]	width of tension flange
Ac	$A_c$	[mm <sup>2</sup> ]	gross area of concrete section
zc2	$z_{c2}$	[mm]	distance of CGS from top fibre

#### Laststellung und Geometrie (loading and geometry)

aa	$a_a$	[in → mm]	dimension of support plate
af	$a_f$	[in → mm]	dimension of loading plate

ba	$b_a$	[in → mm]	distance between support axis and beam end
L	L	[in → mm]	span
c	c	[in → mm]	distance between point loads
a	a	[in → mm]	distance of point load from support axis
kap	a/d	[ - ]	moment - shear force - ratio

Längsbewehrung (Zugbewehrung) (longitudinal tension reinforcement)

cc	$c_c$	[in → mm]	minimum concrete cover
ds	$d_s$	[in → mm]	effective depth of reinforcing bars
Stab_Z	Stäbe	[ - ]	number and diameter of bars
ns	$n_s$	[ - ]	number of bars
dst	$\varnothing_{st}$	[in → mm]	average diameter
fR	$f_r$	[ - ]	r = ribbed bars; 0=plain bars
As	$A_s$	[in <sup>2</sup> → mm <sup>2</sup> ]	area of reinforcing steel for long. reinf.
alphaa	$\alpha_a$	[ - ]	coefficient for anchorage (hook 0.7; straight 1.0; anchor plate 0.01)
rhos	$\rho_s = \frac{A_s}{b \cdot d} \cdot 100$	[ % ]	geometrical reinforcement ratio
rhosw	$\rho_{sw} = \frac{A_s}{b_w \cdot d} \cdot 100$	[ % ]	geom. reinforcement ratio related to web width
fsy	$f_{sy}$	[ksi → MPa]	yield strength of longitudinal steel
esy	$\epsilon_{sy} = \frac{f_{sy}}{E_s}$	[‰]	yield strain (for $E_s = 200.000$ MPa)

ftk	$f_{tk}$	[ksi → MPa]	tensile strength
ftk/fsy	$f_{tk}/f_{sy}$	[ - ]	ratio
euk	$\epsilon_{uk}$	[‰]	steel strain at tensile strength $f_{tk}$

Längsbewehrung (Druckbewehrung) (longitudinal compression reinforcement)

Stab_D	Stäbe	[ - ]	number and diameter of bars
ds2	$d_{s2}$	[in → mm]	distance of compress. reinforc. from compress. edge
ns2	$n_{s2}$	[ - ]	number of compr. bars
dst2	$\varnothing_{st2}$	[in → mm]	average diameter of compr. bars
As2	$A_{s2}$	[in <sup>2</sup> → mm <sup>2</sup> ]	area of compr. bars
fsy2	$f_{sy2}$	[ksi → MPa]	yield strength of compression bars

Bügelbewehrung (stirrups)

dw	$\varnothing_w$	[in → mm]	diameter of stirrups
nsw	$n_{sw}$	[ - ]	number of stirrup legs
Asw	$A_{sw}$	[in <sup>2</sup> → mm <sup>2</sup> ]	area of stirrups
frw	$f_{rw}$	[ - ]	r = ribbed bars; 0=plain bars
sw	$s_w$	[in → mm]	spacing of stirrups in long direction
sw / h	$s_w/h$	[ - ]	ratio
sw / d	$s_w/d$	[ - ]	ratio
zw1	$z_{w1}$	[mm]	inner lever arm defined by stirrup height
zw2	$z_{w2}$	[mm]	inner lever arm defined by concrete cover

rho <sub>w</sub>	$\rho_w = \frac{A_{sw}}{s_w \cdot b_w} \cdot 100$	[%]	geom. percentage of web reinforcement
fy <sub>w</sub>	f <sub>yw</sub>	[ksi → MPa]	yield strength of stirrups
fy <sub>w</sub> _rho <sub>w</sub> _100	$\frac{f_{yw} \cdot \rho_w}{100}$	[MPa]	smeared stirrup forces (stress)
fw <sub>tk</sub>	f <sub>wtk</sub>	[ksi → MPa]	tensile strength of stirrups
fw <sub>tk</sub> /fy <sub>w</sub>	f <sub>wtk</sub> /f <sub>yw</sub>	[ - ]	ratio
ew <sub>uk</sub>	ε <sub>wuk</sub>	[‰]	strain at tensile strength of stirrups

Betondruckfestigkeit (concrete compressive strength)

dia <sub>a</sub>	Ø <sub>a</sub>	[in → mm]	max. diameter of aggregates
f <sub>1c</sub>	f <sub>1c</sub>	[MPa]	uniaxial compr. strength of concrete
Method		[ - ]	test method (cyl; cu; pr)
fc <sub>wu</sub>	f <sub>cwu</sub>	[MPa]	compr. strength of struts in web

Betonzugfestigkeit (concrete tensile strength)

f <sub>1ct, test</sub>	f <sub>1ct, test</sub>	[MPa]	test value for axial tensile strength
Method		[ - ]	test method (fl; sp)
bet <sub>ct, test</sub>	$\beta_{ct, test} = \frac{f_{1ct, test}}{f_{1c}}$	[ - ]	ratio
f <sub>1ct, m, cal</sub>	f <sub>1ct, m, cal</sub>	[MPa]	calculated value of axial concrete tensile strength
bet <sub>ct, cal</sub>	$\beta_{ct, cal} = \frac{f_{1ct, m, cal}}{f_{1c}}$	[ - ]	ratio

mechanischer Bewehrungsgrad (mechanical reinforcement ratio)

oms	$\omega_s = \frac{\rho_s \cdot f_{sy}}{f_{lc} \cdot 100}$	[ - ]	mechanical ratio of longitudinal reinforcement
oml	$\omega_l = \omega_s$	[ - ]	mechanical reinforcement ratio of tension chord
omwy	$\omega_{wy} = \frac{\rho_w}{100} \cdot \frac{f_{yw}}{f_{cwu}}$	[ - ]	mechanical reinforcement ratio of stirrups

Versuch (test)

g	$g = A_s \cdot 24$	[kip/in→kip/ft→kN/m]	calculated self weight
Vg	$V_g = g \cdot (0,5 \cdot c + (a - x_r))$	[kip → kN]	shear force due to self weight
F	F	[kip → kN]	ultimate load
Vu_F+g,Rep	$V_{u,F+g,Rep}$	[kip → kN]	reported shear force at failure with self weight
Vu_Rep	$V_{u,Rep}$	[kip → kN]	shear force at failure with self weight (from report)
Vu_gF	$V_{u,g+f}$	[kip → kN]	shear force at failure with calculated self weight
betar_meas	$\beta_r$	[ ° ]	measured angle of inclined cracks
xr_meas	$x_r$	[ - ]	measured distance of crack from support axis
xr	$x_r$	[ - ]	distance of crack from support axis
sigswmeas	$\sigma_{sw,meas}$	[ksi → MPa]	measured stirrup stress at failure
sigswass	$\sigma_{sw,ass}$	[ksi → MPa]	assumed stirrup stress at failure
sigsw	$\sigma_{sw}$	[ksi → MPa]	stirrup stress at failure
omwu	$\omega_{wu} = \omega_{wy} (\sigma_{sw} / f_{yw})$	[ - ]	mechanical reinforcement ratio of stirrups at failure
br		[ - ]	type of failure
oft		[ - ]	other failure types
bem		[ - ]	remarks

Control criteria

konrect [ - ] rectangular cross-section

Bruchschnittgrößen (moment and shear force at failure)

Mu  $M_u = \frac{V_{u,Rep} \cdot a}{1000}$  [kNm] max. moment at failure

muu  $\mu_u = \frac{M_u}{b \cdot d_s^2 \cdot f_{lc}}$  [ - ] non-dimensional ultimate moment

vutest  $v_{u, test} = \frac{\tau_{u, test}}{f_{cwu}} = \frac{V_{u, Rep}}{b_w \cdot \zeta \cdot d \cdot f_{cwu}}$  [ - ] non-dimensional shear force at failure

Innerer Hebelarm beim Bruch (inner lever arm at failure)

sigs1  $\sigma_{sl} = \frac{1 - \sqrt{1 - 2 \cdot \mu_u / \kappa_c}}{\omega_1} \cdot \kappa_c \cdot f_{sy}$  [MPa] stress in longitudinal reinforcement

xsitest  $\xi_{test} = \frac{\omega_1 \cdot \sigma_{sl}}{\kappa_c \cdot f_{sy}}$  [ - ] coefficient for depth of compr. zone

zetatest  $\zeta_{test} = 1 - \frac{\xi_{test}}{2}$  [ - ] coefficient for inner lever arm

z\_test  $z_{test} = \zeta_{test} \cdot d_s$  [mm] inner lever arm

Kontrolle Biegung (check of flexural capacity)

epcu  $\varepsilon_{cu} = 4 - 2 \cdot \frac{f_{1,c}}{100}$  [ % ] maximum concrete strain

kapc  $\kappa_c = 1 - \frac{f_{lc}}{250}$  [ - ] coefficient for maximum stress of stress block

zeta  $\zeta$  [ - ] coefficient for inner lever arm

$z_{-}$	$z = d_s \cdot \zeta$	[ mm ]	inner lever arm
$\mu_{flex}$	$\mu_{u,flex}$	[ - ]	dimensionless moment at flexural failure
$Mu_{flex}$	$M_{u,flex}$	[ kNm ]	moment at flexural failure
$Vu_{flex}$	$V_{u,flex}$	[ kN ]	shear force at flexural failure
$\beta_{flex}$	$\beta_{flex} = \frac{\mu_u}{\mu_{flex}}$	[ - ]	ratio
<u>Querkraft – PT (für <math>\nu = 0,8</math>) (ultimate shear force acc. to PT (for <math>\nu = 0,8</math>))</u>			
$\sin 2\theta_p$	$\sin^2 \theta_p$	[ - ]	
$\theta_p$	$\theta_p = \arcsin \sqrt{\sin^2 \theta_p} \cdot \frac{180}{\pi}$	[ ° ]	angle $\theta$ acc. to PT (measured against long axis)
$\cot \theta_p$		[ - ]	
$v_{up}$	$v_{up} = \omega_{wy} \cdot \cot \theta_p$	[ - ]	ultimate non-dimensional shear force acc. to PT
<u>Bruchquerkraft im Versuch (ultimate shear force at failure )</u>			
$\gamma_{wp}$	$\gamma_{wp} = \frac{v_{u,test}}{v_{up}}$	[ - ]	model safety factor
$\cot \theta_u$	$\cot \theta_u = \frac{v_{u,test}}{\omega_{wu}}$	[ - ]	$\cot \theta$ at failure
$\theta_u$	$\theta_u = \arctan \left( \frac{1}{\cot \theta_u} \right) \cdot \frac{180}{\pi}$	[ ° ]	$\cot \theta$ at failure
$\nu_u$	$\nu_u = \frac{(0,8 \cdot \omega_{wu})}{\sin^2(\theta_u \cdot \pi/180)}$	[ - ]	coefficient for effective strength ( $\nu_u = f_{cwu}/f_{lc}$ )

Verankerung (check of anchorage at end support)

betalb	$\beta_{lb} = \frac{l_{berf}}{l_{bvorh}}$	[ - ]	ratio
--------	---	-------	-------

Druckfestigkeiten für Auswertungen (strength values for evaluations)

f <sub>lck</sub>	$f_{lck} = f_{lc} - 3,8$	[MPa]	characteristic uniaxial compressive strength
f <sub>cm_cyl</sub>	$f_{cm,cyl} = f_{lc} / 0,95$	[MPa]	mean cylinder strength
f <sub>ck</sub>	$f_{ck} = f_{cm,cyl} - 4,0$	[MPa]	characteristic concrete cylinder strength
f <sub>c_prime</sub>	$f'_c = f_{ck} + 1,6$	[MPa]	characteristic cylinder strength of ACI 318